BKF
ENGINEERS
SURVEYORS
PLANNERS

Mr. Gustavo Gomez
City of Santa Clara
Engineering Department
1500 Warburton Avenue
Santa Clara, CA 95050

July 20, 2015

Subject: **Building G Sewer Study**
3333 Scott Campus Development (3535 Garrett Drive)

Dear Mr. Gomez,

This report will analyze the sanitary sewer flows for the proposed development at the project site located at 3535 Garrett Drive. The proposed site consists of an 8-story office building (Building G) and a parking structure (Parking Structure P2). The existing 118,800 square foot building structure that is currently on the 3535 Garrett site is an occupied building. It will be demolished and replaced with the new Building G. Building G accounts for 247,769 square feet of office space. Parking Structure P2 will not contribute to the sanitary sewer flows for the site. For the purpose of this sanitary sewer study, the square footage of the existing occupied building will be credited to (subtracted from) the proposed Building G's office space square footage.

Proposed Building G Area	**Existing (Occupied) 3535 Garrett Drive Building Area**	**Total Area to be Analyzed**
247,769 sq. ft.	118,800 sq. ft.	**128,969 sq. ft.**

The Building G square footage that will be contributing additional sewer flow to the Tannery Way sanitary sewer system will be 128,969 square feet.
Please refer to Exhibit "B" for the Scott Campus Site Plan detailing the existing and proposed building square footage.

The Sanitary Sewer Report by V&A dated September 2011 documents the sanitary sewer flow at Tannery Way, upstream of Sanitary Sewer Manhole 13 as shown on the Sanitary Sewer Exhibit "A". This report includes sanitary sewer flows from existing buildings along Scott Boulevard, Garrett Drive (including 3535 Garrett Drive), and Tannery Way that fall within the tributary area for Sanitary Sewer Manhole 13.

Sanitary Sewer Calculation Sheet for Site 1 (See exhibit for Location)

Block Maps S72 and S73 provided by the City of Santa Clara (see attached) show the Tributary Area served by the sanitary sewer main that was monitored by V&A. The total project Tributary Area is approximately 64.22 acres. The City of Santa Clara Sanitary Sewer Capacity Assessment Figure 3-3 shows the flow meter areas used for infiltration and inflow analysis. Approximately 10.10 acres (15.7%) of the Tributary Area falls within meter area M_07. Of that area, 4.74 acres are part of the project site and 5.36 acres are from the Tributary Area surrounding the site. The remainder of the Tributary Area,

1650 Technology Drive
Suite 650
San Jose
California 95110
phone 408.467.9100
fax 408.467.9199
www.bkf.com

54.12 acres (84.3%), falls within meter area M_08. The breakdown of the tributary areas is shown in Exhibit "A".

The formula below was used to calculate the flow contributing to the Garrett/Tannery System:

$Q_D = Q_M + Q_{WWGWI} + Q_{RDI/I} + Q_{PD}$

Monitored Flow

Q_M = The greater of:
Monitored Peak Flow **-Or-** 2.5 x (Monitored Average Flow)

From V&A Sanitary Sewer Report, dated September 2011 (attached):
Monitored Peak Flow = 14.0 gpm
Monitored Average Flow = 5.8 gpm
2.5 x (Monitored Average Flow) = 2.5 x 5.8 gpm = 14.5 gpm > 14.0 gpm

Q_M = **14.5 gpm**

Wet Weather Groundwater Infiltration

Per the City of Santa Clara Sanitary Sewer Capacity Assessment Table 3-2 GWI and RDI/I Parameters by Meter Area (attached), the Wet Weather GWI factor for meter area M_07 is 1,900 gpd/acre, and for meter area M_08, the factor is zero. The calculation for the weighted Wet Weather GWI rate is shown below.

Q_{WWGWI} = 1,900 gpd/Acre

Tributary Area (TA) for Meter Area M_07 with the 12" SS:

3385 Scott Boulevard:	2.03 Acres
3375 Scott Boulevard:	2.52 Acres
Garrett Drive R/W Area:	0.36 Acres (Portion of Garrett Drive roadway TA that is within M_07)
APN 216-31-060:	0.45 Acres (Portion of TA within M_07)
APN 216-31-080:	4.74 Acres (Portion of Project Site within M_07)

Total M_07 Tributary Area = 10.10 Acres
(See Sanitary Sewer Exhibit "A" for Tributary Areas)

Q_{WWGWI} = (1,900 gpd/Acre) x (10.10 Acres)
= **19,190 gpd or 13.33 gpm**

Rainfall-Dependent Infiltration and Inflow

Per the City of Santa Clara Sanitary Sewer Flow Monitoring Design Flow Determination worksheet, the Rainfall-Dependent Infiltration and Inflow factor for both meter areas M_07 and M_08 is 1,000 gpd/acre. The calculation for the Rainfall-Dependent Infiltration and Inflow rate is shown below.

$Q_{RDI/I}$ = (1,000 gpd/Acre) x (64.22 Acres) = **64,220 gpd or 44.60 gpm**

Proposed Development Peak Flow

The City of Santa Clara Sanitary Sewer Capacity Assessment Table 2-5, Base Wastewater Flow Unit Flow Factors for Office/R&D Type Development shows:

Unit Flow Factor = 0.15 gpd/Sq. Ft.
Peaking Factor = 2.5

Vacant Building Areas on Adjacent Lots

APN 216-31-062	= 39,182 Sq. Ft.
APN 216-31-070	= 36,895 Sq. Ft.
APN 216-31-069	= 43,756 Sq. Ft.
APN 216-31-068	= 82,633 Sq. Ft.
APN 216-30-048	= 52,011 Sq. Ft.
APN 216-30-049	= 72,435 Sq. Ft.
Total Area:	= 326,912 Sq. Ft.

Proposed Development:

Building G	247,769 Sq. Ft → Discharges to Tannery Way
Existing 3535 Garrett Bldg	-118,800 Sq. Ft. → Included in V&A Flow Analysis
Total Net Area:	**128,969 Sq. Ft. (To be analyzed in this report.)**

Q_{PD} = 2.5 x [(Unit Flow Factor) x (Total Net Area Square Footage) +
(Unit Flow Factor) x (Total Vacant Building Areas Square Footage)]

Q_{PD} = 2.5 x [(0.15 gpd/Sq. Ft.) x (128,969 Sq. Ft.) +
(0.15 gpd/Sq. Ft.) x (326,912 Sq. Ft.)]
= 2.5 x [19,346 gpd + 49,037 gpd] = 170,956 gpd
= 118.72 gpm

$Q_D = Q_M + Q_{WWGWI} + Q_{RDI/I} + Q_{PD}$

Q_D= 14.5 gpm + 13.33 gpm + 44.60 gpm + 118.72 gpm

Q_D= 191.15 gpm = 0.43 CFS

July 20, 2015
Job No: 19976093
Gustavo Gomez

Pipe Capacity of the 12" Sanitary Sewer at 0.246% [1] = **1.78 CFS = 799 gpm**

From Manning's Equation (See attached Sanitary Sewer Calculations for Site 1):

Depth of Flow of 12" SS at 0.246% [1] = 0.33 ft

Depth of Flow/Pipe diameter = (0.33 ft/1 ft) = 0.33 < 0.75

Pipe is adequate for proposed development

[1] Slope of sanitary sewer pipe per survey

Please contact me if you have any questions or require any additional information.

Sincerely,

Nellie Moussa, P.E.
Project Engineer

Sanitary Sewer Calculations for Site 1

Project Name: Building G – 3535 Garrett Drive (3333 Scott Site)

BKF Project No: 19976093

Date: 07/20/2015

Manning Pipe Calculator

Given Input Data:

Shape ..... Circular

Solving for ..... Depth of Flow

Diameter ..... 1.0000 ft

Flowrate ..... 0.4259 cfs ←Q_D

Slope ..... 0.0025 ft/ft

Manning's n 0.0130

Computed Results:

Depth ..... 0.3329 ft ← Depth of Flow

Area 0.7854 ft2

Wetted Area ..... 0.2287 ft^2

Wetted Perimeter ..... 1.2300 ft

Perimeter ..... 3.1416 ft

Velocity ..... 1.8621 fps

Hydraulic Radius ..... 0.1860 ft

Percent Full ..... 33.2858 %

Full flow Flowrate ..... 1.7814 cfs

Full flow velocity ..... 2.2682 fps

P.M. 429–M–11

TRIBUTARY AREA

PCL. 1 **68**

APN 216–31–068

5.42 AC.NET.

PETERSON WAY

P. M. 415 –M– 24

APN 216–30–049

TRIBUTARY AREA

APN 216–30–048

MANHOLE 13

TANNERY WAY

WAY

1"=200'

Creek

PCL. 2 **69**

APN 216–31–069

2.87 AC.NET.

DRIVE

77

APN 216–31–077

PCL.2

5.69 AC.

PCL. 1

5.835 AC.

78

APN 216–31–078

PCL. 2

9.669 AC.

79

APN 216–31–079

67

APN 216–31–067

4.11 AC. NET.

PCL. B

DRIVE

LEGEND

METER AREA M_07

TRIBUTARY AREA SERVED BY SEWER MAIN TO MANHOLE 13

Calabazas

PCL. 3 **70**

APN 216–31–070

2.42 AC.NET.

PCL. 3

2.57 AC.NET.

62

APN 216–31–062

S.C.V.W.D.

GARRETT

PCL. 2

3.27 AC.NET.

60

APN 216–31–060

PCL. 3

10.559 AC.

APN 216–31–080

PCL. 4

4.152 AC.

APN 216–31–081

81

TRIBUTARY AREA

P.M.456–M–56

AUGUSTINE

DRIVE

72 0.51AC. NET.

PCL. B

71

1.03AC. NET.

CITY OF SANTA CLARA

32

1.32 AC.NET.

PCL. A

PCL. 1

2.44 AC. NET.

48

AVENUE

LAKESIDE

BOWERS

CEN. SE T. 6 S. - R

3385 SCOTT BLVD.
APN 216–31–063
2.03 AC.NET.
63

3375 SCOTT BLVD.
APN 216–31–059
2.52 AC.NET.
59

80

PORTION OF
3333 SCOTT BLVD. WITHIN
METER AREA M_07
4.74 AC.

TRIBUTARY AREA

P. M. 399 –M– 44, 45, 46

SCOTT BOULEVARD

P.M. 789–M–3

P.M. 369–M–22,23

PORTION OF GARRETT DRIVE R/W WITHIN METER AREA M_07
0.36 AC.

PORTION OF APN 216–31–060 WITHIN METER AREA M_07
0.45 AC.

BKF ENGINEERS | SURVEYORS | PLANNERS
1650 TECHNOLOGY DRIVE
SUITE 650
SAN JOSE, CA 95110
408–467–9100
408–467–9199 (FAX)

3333 SCOTT BOULEVARD
SANITARY SEWER EXHIBIT "A"
TRIBUTARY AREAS

SANTA CLARA

CALIFORNIA

SANTA CLARA

No.	Revisions

Date: 9–30–11
Scale: 1" = 200'
Design: NH
Drawn: NH
Approved: MBP
Job No: 19976093

Drawing Number:

EXH A

1 OF 1

DRAWING NAME: K:\ENG\97\76093\Sanitary Sewer Report\Sanitary Sewer Exhibit.dwg
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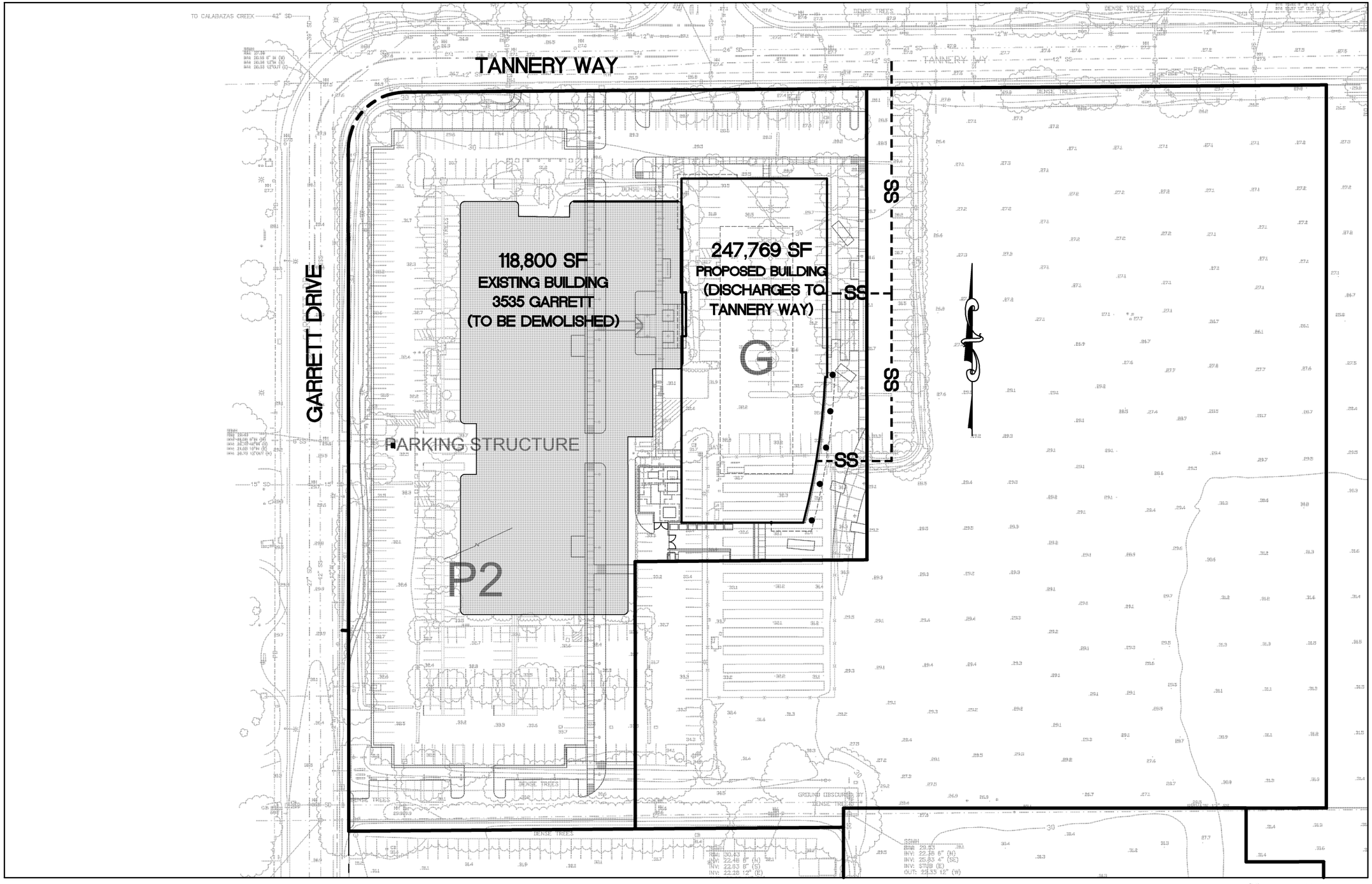
PLOTTED BY: hora



MENLO EQUITIES

DATE: 07-20-2015

3333 SCOTT BLVD, SANTA CLARA, CA

SANITARY SEWER SYSTEM
EXHIBIT B

BKF
1650 TECHNOLOGY DRIVE, SUITE 650
SAN JOSE, CA 95110
408-467-9100
WWW.BKF.COM

PLAN HUNTER
RMW
architecture & interiors
vision
function
space
culture
ideas
experience
listen
build
speak
learn

EX-B

CITY OF SANTA CLARA

BASIC INFORMATION MAP

100 50 0 100 200 300 400 500 FT.

SCALE

N W E S

DATE LAST CORRECTED 12-2008

MAP NO. S 72

N 326,750

E 1,567,550

82

71

73

62

N 323,500

E 1,569,850

HIGHWAY 101 (BAYSHORE FREEWAY)

Lakeway Wy.

Lakeside Dr.

CALABAZAS CREEK

Peterson Wy.

Oakmead Parkway

SUNNYVALE CITY LIMIT

SANTA CLARA CITY LIMIT

TRIBUTARY AREA

20.33

19.40

Tannery Wy.

Oakmead III

AD 171

Garrett Dr.

12" VCP-PRIVATE MAIN (S.S. CORP.)

C/O TO C/O

CITY S.C. C/O AT P/L

Arques Ave.

SCOTT BLVD.

Oakmead Village Dr.

PRIVATE LINE

S 72

CITY OF SANTA CLARA

BASIC INFORMATION MAP

100 50 0 100 200 300 400 500 FT.

SCALE

N W E S

DATE LAST CORRECTED

12-2008

MAP NO.

S 73

N 326,750

E 1,569,850

83

72

74

63

MISSION COLLEGE BLVD.

HIGHWAY 101

GREAT AMERICA PARKWAY

(BAYSHORE FREEWAY)

Lakeside dR.

Tannery Wy.

Lakeside Dr.

Augustine Dr.

BOWERS AVE.

SCOTT BLVD.

SCOTT BLVD.

Coronado Dr.

TRIBUTARY AREA

FLOW METER LOCATION

12"VCP-PRIVATE LINE (I.S.S. CORP.) — C/O TO C/O —

CITY S.C. C/O AT P/L

E 1,572,150

N 323,500

S 73

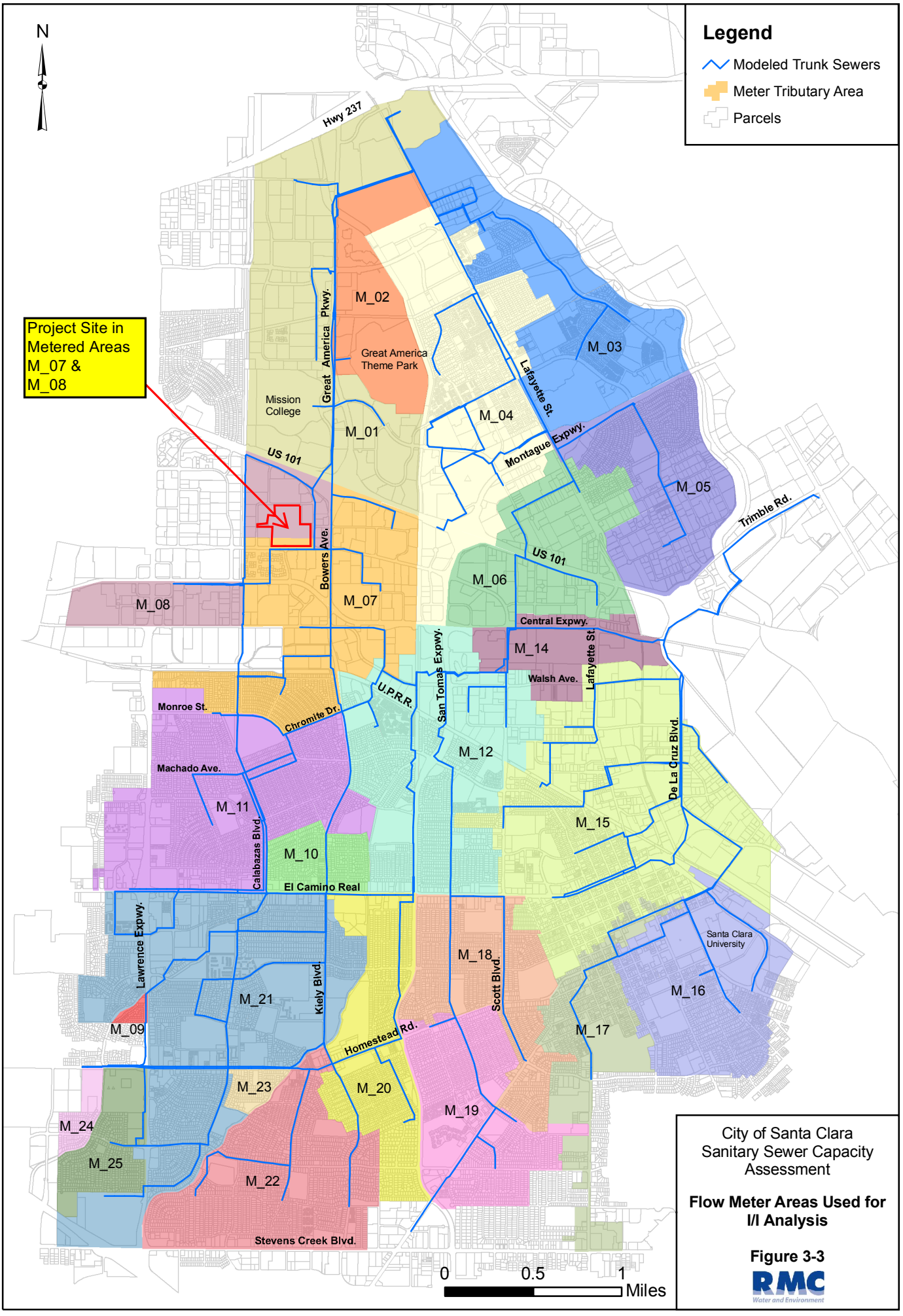

N
Legend
Modeled Trunk Sewers
Meter Tributary Area
Parcels
Project Site in Metered Areas M_07 & M_08
Hwy 237
M_02
Great America Pkwy.
Great America Theme Park
Mission College
M_01
M_03
Lafayette St.
M_04
Montague Expwy.
M_05
Trimble Rd.
US 101
Bowers Ave.
M_07
M_08
M_06
US 101
Central Expwy.
M_14
Lafayette St.
Walsh Ave.
San Tomas Expwy.
U.P.R.R.
Monroe St.
Chromite Dr.
Machado Ave.
M_12
De La Cruz Blvd.
M_11
M_15
Calabazas Blvd.
M_10
El Camino Real
Lawrence Expwy.
Santa Clara University
M_18
Scott Blvd.
Kiely Blvd.
M_21
M_16
M_09
Homestead Rd.
M_17
M_23
M_20
M_19
M_24
M_25
M_22
Stevens Creek Blvd.
0
0.5
1
Miles
City of Santa Clara
Sanitary Sewer Capacity Assessment
Flow Meter Areas Used for I/I Analysis
Figure 3-3
RMC
Water and Environment

Table 3-2 GWI and RDI/I Parameters by Meter Area

Meter Area[a]	Dry Weather GWI[b] (gpd/acre)	Wet Weather GWI[c] (gpd/acre)	R1 RDI/I Vol. (%) (2 hrs. to peak)	R2 RDI/I Vol. (%) (6 hrs. to peak)	R3 RDI/I Vol. (%) (12 hrs. to peak)
M_01	0	0	0.5	0.8	0.8
M_02	0	0	0.5	0.8	0.8
M_03	0	0	0.6	0.1	0.1
M_04	500	1,300	0.6	0.1	0.1
M_05	700	1,000	0.6	0.1	0.1
M_06	0	0	0.6	0.1	0.1
M_07	1,900	1,900	0.3	0.5	0.5
M_08	0	0	0.3	0.5	0.5
M_09	0	0	0.6	0.1	0.1
M_10	0	0	0.6	0.1	0.1
M_11	1,600	2,300	0.9	1.7	6.0
M_12	0	0	0.9	1.0	0.5
M_14	0	0	0.6	0.1	0.1
M_15	300	700	1.0	0.2	0.2
M_16	900	1,600	1.0	0.2	0.2
M_17	200	200	0.6	0.1	0.1
M_18	0	0	0.8	1.0	0.1
M_19	0	0	0.3	0.1	0.1
M_20	0	0	0.6	0.1	0.1
M_21	0	0	0.6	0.1	0.1
M_22	0	0	0.6	0.1	0.1
M_23	0	0	0.6	0.1	0.1
M_24	0	0	0.6	0.1	0.1
M_25	0	0	0.6	0.1	0.1
CuSD	0	0	0.5	0.2	0.4

(a) See Figure 3-3.
(b) Represents GWI during non-rainfall periods (e.g., early to mid-February) of the 2006 flow monitoring period.
(c) Represents GWI immediately following rainfall events.

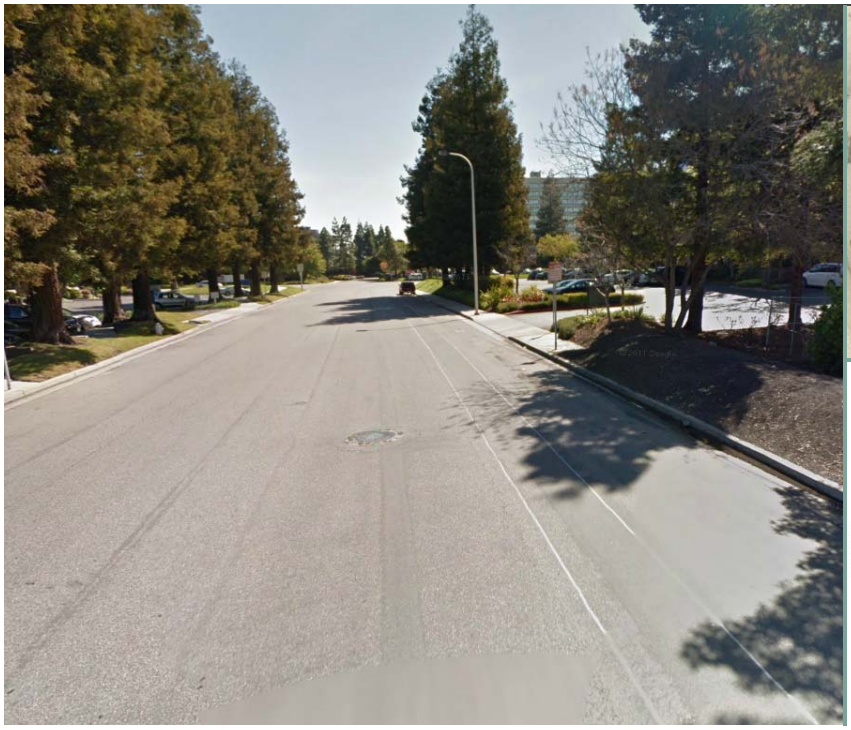
Table 2-5 Base Wastewater Flow Unit Flow Factors

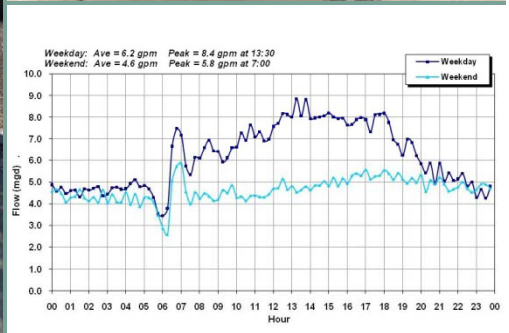
Type of Development	Unit Flow Factor	Basis
Single Family Detached	245 gpd/DU	3.5 people/DU @ 70 gpcd
Townhouses/Condominiums	175 gpd/DU	2.5 people/DU @ 70 gpcd
Apartments	154 gpd/DU	2.2 people/DU @ 70 gpcd
Hotels	100 gpd/room	
Commercial/Office	0.1 gpd/sq. ft.	
Office/R&D	0.15 gpd/sq. ft.	
Moderate Density Residential (Mixed Use)	3,200 gpd/acre	21 DU/acre @154 gpd/DU
Medium Density Residential (Transit-Oriented Mixed Use)	4,600 gpd/acre	30 DU/acre @ 154 gpd/DU
Commercial/Office/R&D Intensification[a]	+ 300 gpd/acre	+ 0.04 FAR @ 0.15 gpd/sq. ft.

(a) Applied to areas of North Santa Clara where existing development is anticipated to increase in intensity from a current average floor-area-ratio (FAR) of 0.41 to a future average of 0.45.

2.3.3 Diurnal Base Wastewater Flow Patterns

In most sewer systems, BWF exhibits typical diurnal patterns depending on the type of land use. For Santa Clara, typical diurnal curves were developed for residential, commercial, and industrial areas, for both weekend and weekday conditions. These curves are shown in **Figure 2-4**. Each area of the system was assigned a diurnal curve according to its predominant land use type.





SANITARY SEWER FLOW MONITORING AND CAPACITY ANALYSIS

City of Santa Clara

September 2011

V&A

SANITARY SEWER FLOW MONITORING AND CAPACITY ANALYSIS

City of Santa Clara

Prepared for:
Menlo Equities
490 California Street, 4th Floor
Palo Alto, CA 94306

Prepared by

V&A

September 2011

TABLE OF CONTENTS

TABLES

FIGURES

EXECUTIVE SUMMARY

V&A has completed a sanitary sewer flow monitoring and capacity study within the City of Santa Clara, CA. One 12-inch segment on Tannery Way (Figure 2) was monitored for a one-week period from August 25, 2011 to September 6, 2011. The purpose of this study was to capture existing baseline flows at the flow monitoring location. The results of flow monitoring are summarized in Table 1. Figure 1 shows graphical snapshots of average and peak measure flows. The flow monitoring data suggests that there is a pump station upstream from the flow monitoring site. Given a typical pump station's run time and the flow monitoring data reporting interval of 15 minutes, the instantaneous peak flows discharging the pump station may not have been captured at through the flow monitoring site due to attenuation.

Table 1. Summary of Flow Monitoring Data

Item	Result
Site 1 – Estimated 100% Capacity of Pipeline:	796 gpm
Site 1 – Average Dry Weather Flow:	5.8 gpm
Site 1 – Peak Measured Dry Weather Flow:	14.0 gpm
Site 1 – Available Capacity (Capacity less Peak):	782 gpm

gpm = gallons per minute

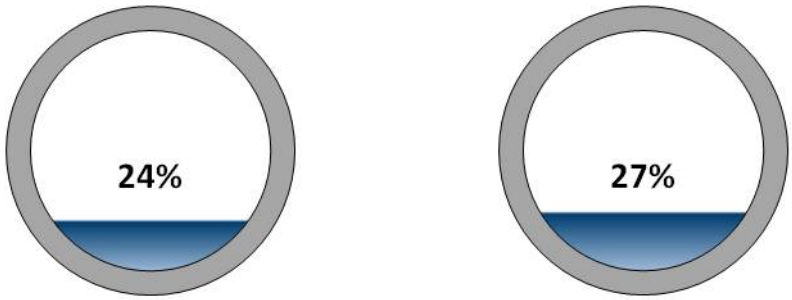


Figure 1. Average and Peak Flow Cross-Sectional Snapshot Views

INTRODUCTION

V&A has completed sanitary sewer flow monitoring for Menlo Equities. Flow monitoring occurred over a one-week period from August 25 to September 6, 2011, at one open-channel flow monitoring site located on Tannery Way in the City of Santa Clara (City), California. The purpose of this study was to capture existing baseline flows at the flow monitoring location.

Figure 2 shows the overview of the flow monitoring location. Photo 1 shows a surface view and Photo 2 shows a plan view of the flow monitoring site. Figure 3 shows the location of the site on a sanitary sewer map and Figure 4 is a flow sketch of the site.

Figure 2. Overview of Flow Monitoring Location

Photo 1. Street View of Flow Monitoring Site

Photo 2. Plan View of Flow Monitoring Site

Figure 3. Sanitary Map

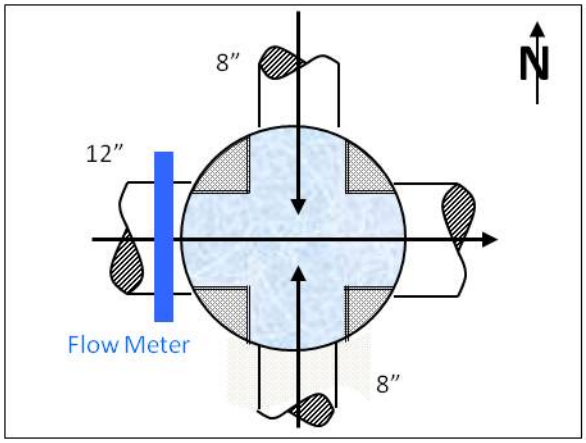


Figure 4. Flow Sketch

METHODS AND PROCEDURES

Confined Space Entry

A confined space is defined as any space that is large enough and so configured that a person can bodily enter and perform assigned work, has limited or restricted means for entry or exit and is not designed for continuous employee occupancy. The atmosphere must be constantly monitored for sufficient levels of oxygen (19.5 to 23.0%) and the absence of hydrogen sulfide (H_2S) gas, carbon monoxide (CO) gas and LEL (lower explosive limit) levels. A typical confined space entry crew has members with OSHA-defined responsibilities of Entrant, Attendant and Supervisor (Photo 3). The Entrant is the individual performing the work. He or she is equipped with the necessary personal protective equipment needed to perform the job safely, including a personal 4-gas monitor (Photo 4). If it is not possible to maintain line-of-sight with the Entrant, then more Entrants are required until line-of-sight can be maintained. The Attendant is responsible for maintaining contact with the Entrant(s) to monitor the atmosphere on another 4-gas monitor and maintaining records of all Entrants, if there is more than one. The Supervisor develops the safe work plan for the job at hand prior to entering.

Photo 3. Confined Space Entry

Photo 4. Typical Personal 4-Gas Monitor

Flow Meter Installation

One Isco 2150 flow meter was installed by V&A. Isco 2150 meters use a pressure transducer to collect depth readings and ultrasonic Doppler sensors on the probe to determine the average fluid velocity. Figure 5 shows a sketch of a typical flow meter installation.

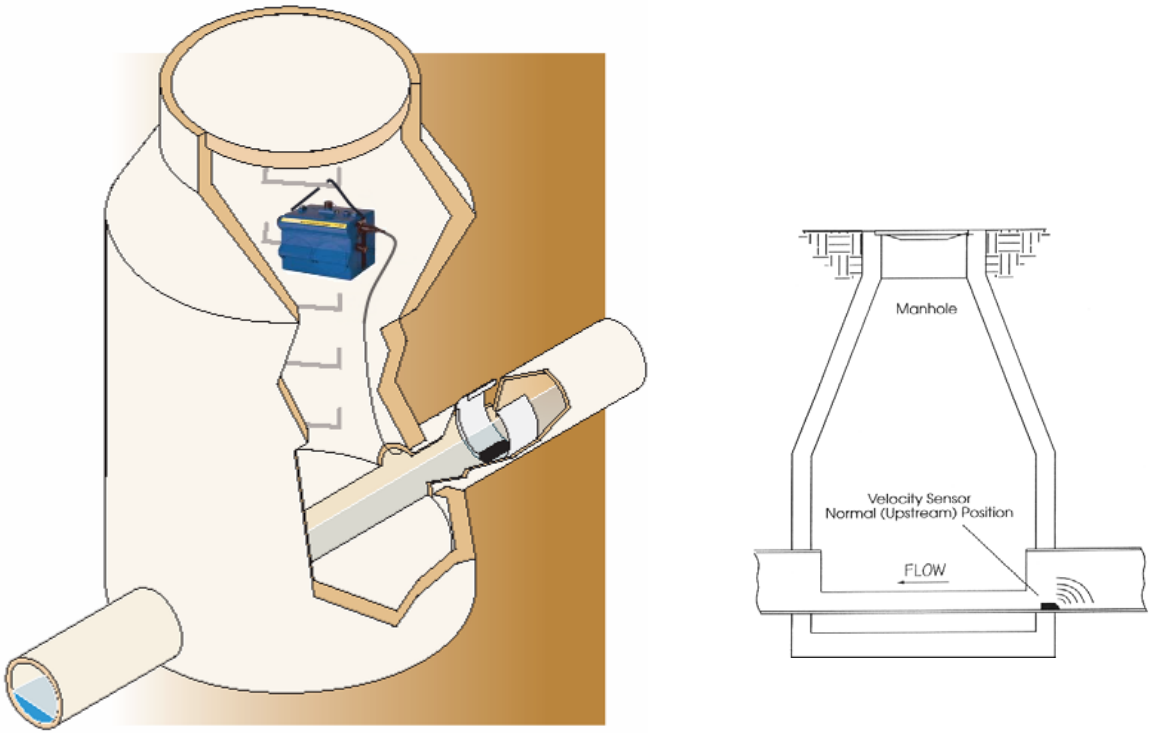


Figure 5. Typical Flow Meter Installation

Continuous depth and velocity readings were recorded by the flow meter in 5-minute increments and downloaded into a computer spreadsheet program where the data could be analyzed and made report ready. Manual level and velocity measurements were taken in the field during flow meter installation and again when the flow meter was removed. These manual measurements were compared to simultaneous level and velocity readings from the flow meter to ensure proper calibration and accuracy. The pipe diameter was also verified in order to use the correct geometry in calculating flows.

FLOW MONITORING RESULTS

Table 2 summarizes the flow monitoring data recorded during this study. Figure 6 and Figure 7 on the following pages graphically illustrate the flow monitoring data recorded during this study. Figure 8 illustrates a snapshot of the pipe full (d/D) condition during average and peak dry weather flow conditions. Note that:

- ADWF = Average Dry Weather Flow,
- Peak = Peak flow as measured during the entire flow monitoring period,
- Peaking Factor = Peak Measured Flow / ADWF,
- d/D Ratio = Peak measured depth / Pipe diameter. The value shown is the highest d/D ratio that was measured during the entire flow monitoring period.

The flow monitoring data suggests that there is a pump station upstream from the flow monitoring site. Given a typical pump station's run time and the flow monitoring data reporting interval of 15 minutes, the instantaneous peak flows discharging the pump station may not have been captured at through the flow monitoring site due to attenuation.

Table 2. Flow Monitoring Results Summary

Item	Result
ADWF – Weekday:	6.2 *gpm*
ADWF – Weekend:	4.6 *gpm*
ADWF – Overall:	5.8 *gpm*
Weekend-to-Weekday Ratio:	0.75
Peak Measured Flow:	14.0 *gpm*
Peaking Factor:	2.4
Peak Flow Depth:	3.3 *inches*
d/D Ratio:	0.27

gpm = gallons per minute

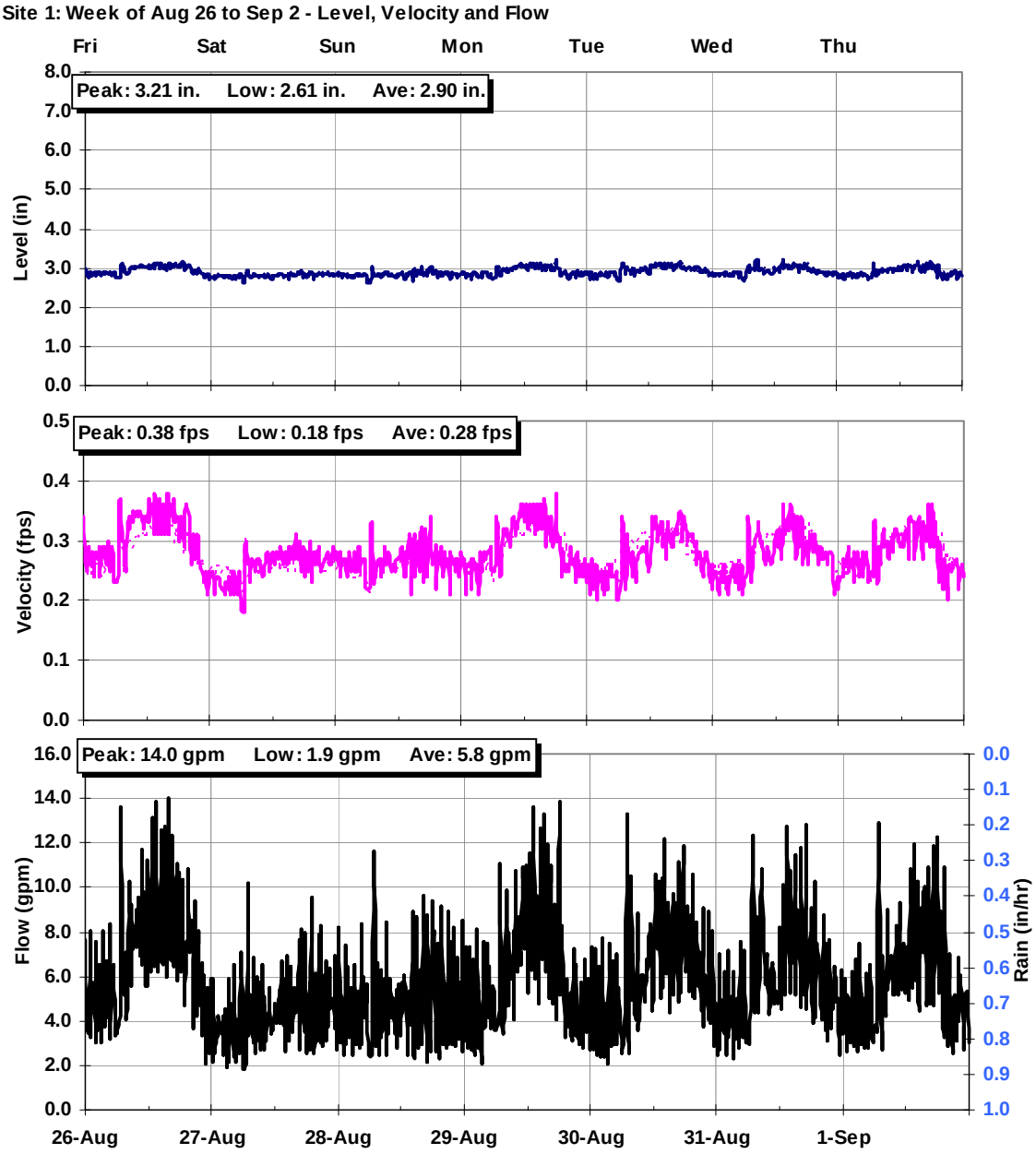


Figure 6. Level, Velocity and Flow Data – August 26 through September 1, 2011

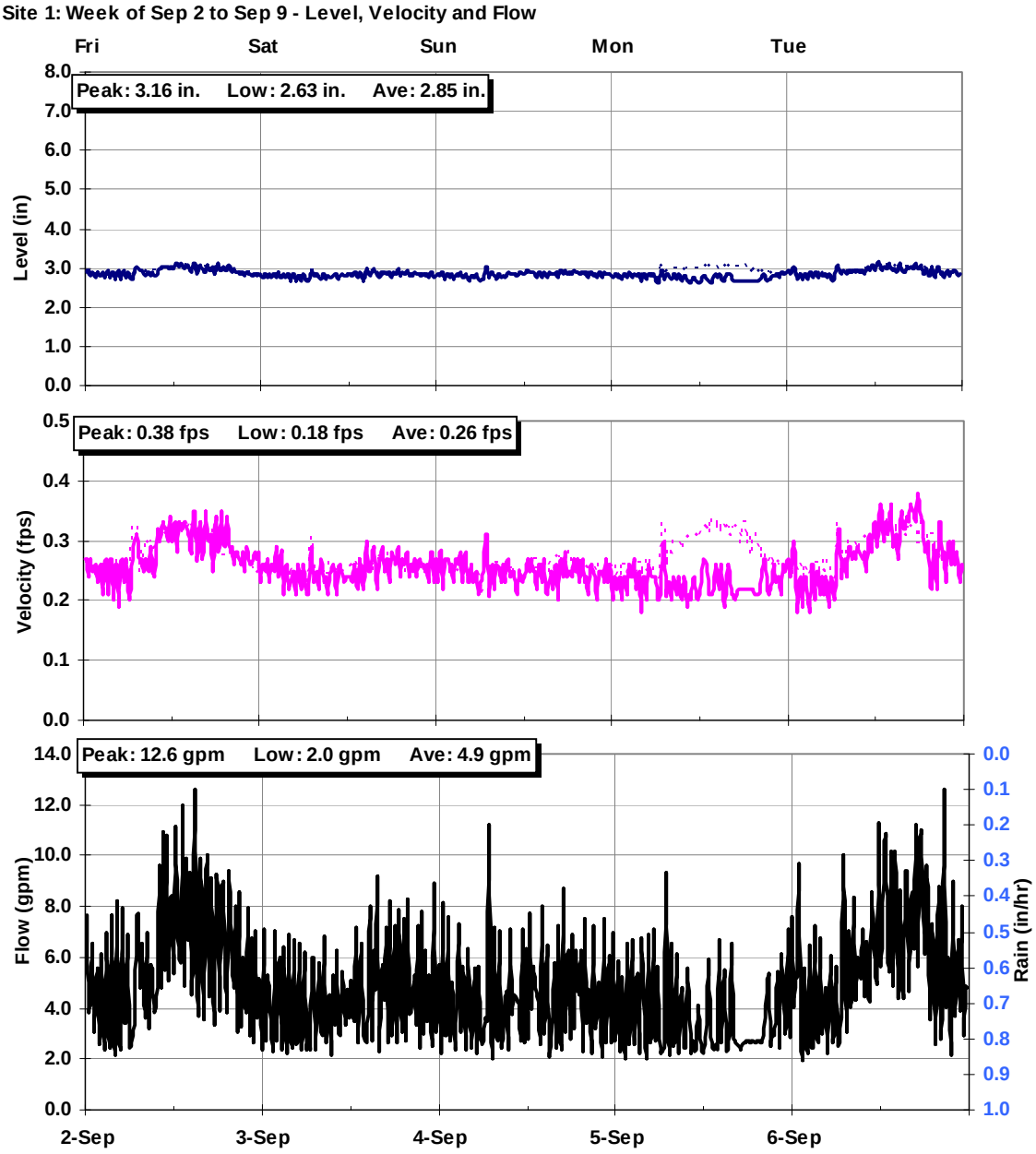


Figure 7. Level, Velocity and Flow Data – September 2 through September 6, 2011

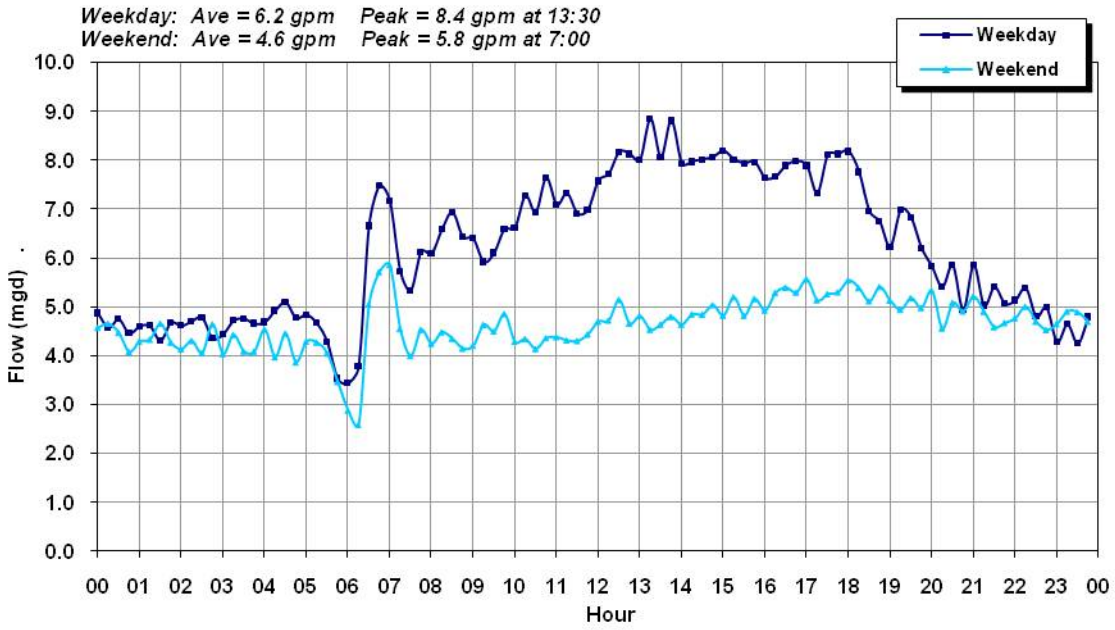


Figure 8. Weekday/Weekend Average Dry Weather Flows

PIPELINE CAPACITY

The pipeline capacity was estimated by using the Manning formula. The Manning formula estimates the hydraulic properties of flow in an open channel based on the following characteristics of the flow conduit:

D = Diameter of pipe (in.)
d = Depth of flow (in.)
S = Pipeline slope
n = Roughness coefficient

The roughness coefficient is assumed to be *n* = 0.013 for sanitary sewer pipe. The capacity of the pipeline is calculated during the full-flow condition wherein the depth of flow is equal to the diameter of the pipeline. For this condition, the capacity of the pipeline in gallons per minute (gpm) is estimated as follows:

$$Capacity = \frac{0.8865 \times \frac{\pi}{4} D^2 \times \left(\frac{D}{4}\right)^{2/3} \times Slope^{1/2}}{n}$$

The slope for the flow monitoring site was obtained from drawings provided by BKF and is 0.246%.

Table 3 summarizes the measured flow and estimated capacity data, including the average dry weather flow and peak measured flow.

Table 3. Average Dry Weather Flow and Peak Measured Flow

Item	Result
Site 1 – Estimated 100% Capacity of Pipeline:	796 gpm
Site 1 – Average Dry Weather Flow:	5.75 gpm
Site 1 – Peak Measured Dry Weather Flow:	13.99 gpm
Site 1 – Available Capacity (Capacity less Peak):	782 gpm

Figure 9 shows average and peak flow cross-sectional view snapshots for the flow monitoring site.

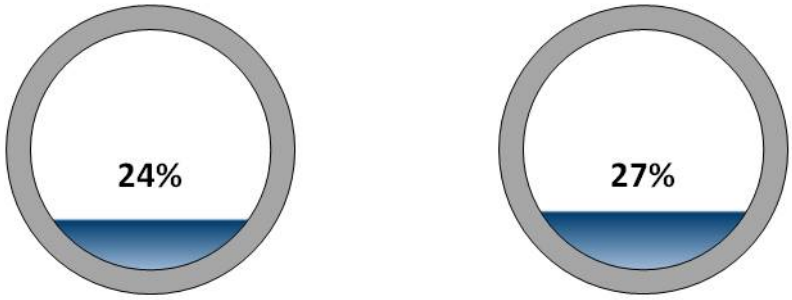
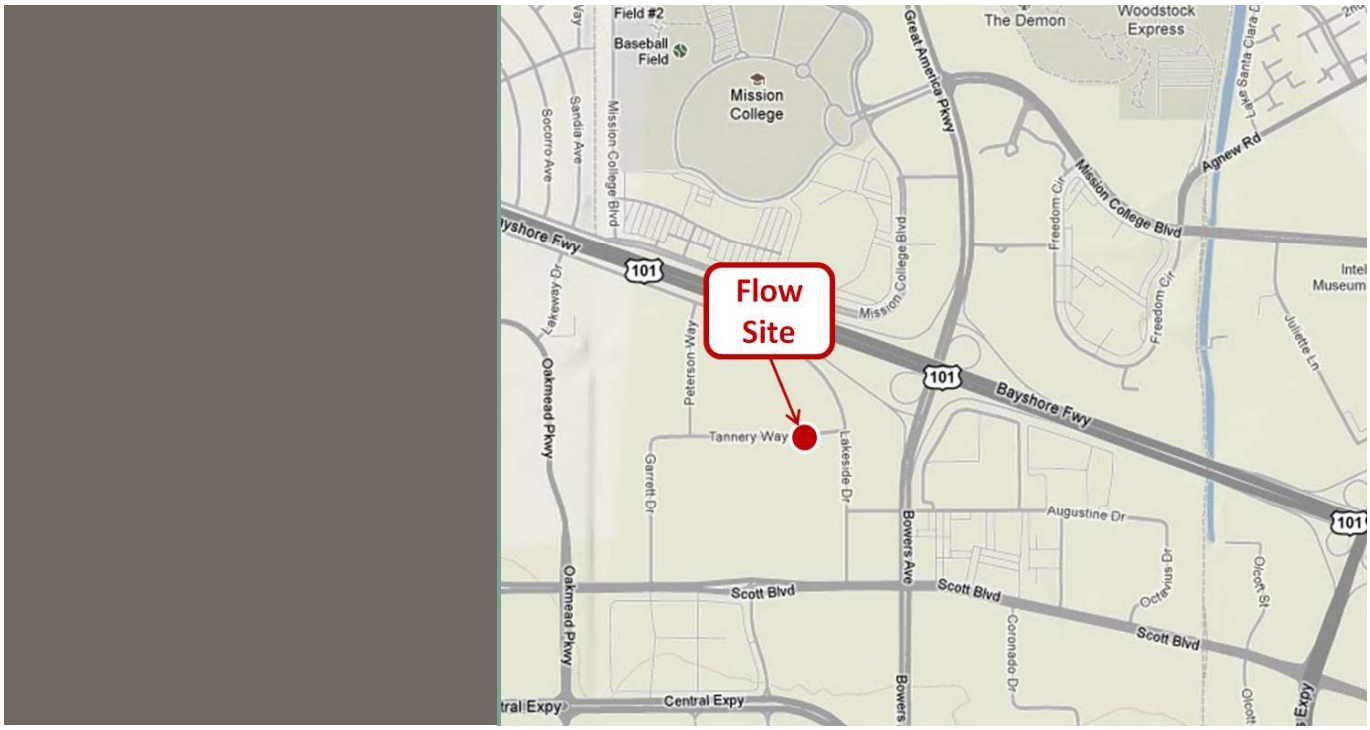


Figure 9. Average and Peak Flow Cross-Sectional Snapshot Views



V&A

Oakland
155 Grand Avenue, Suite 700
Oakland, CA 94612
510.903.6600 **Tel**
510.903.6601 **Fax**

San Diego
8291 Aero Place, Suite 110
San Diego, CA 92123
858.576.0226 **Tel**
858.576.0004 **Fax**

Seattle
14900 Interurban Avenue, Suite 268
Seattle, WA 96818
206.674.4560 **Tel**
206.674.4561 **Fax**

Houston
8220 Jones Road, Suite 500
Houston, TX 77065
713.840.6490 **Tel**
713.840.6491 **Fax**

vaengineering.com